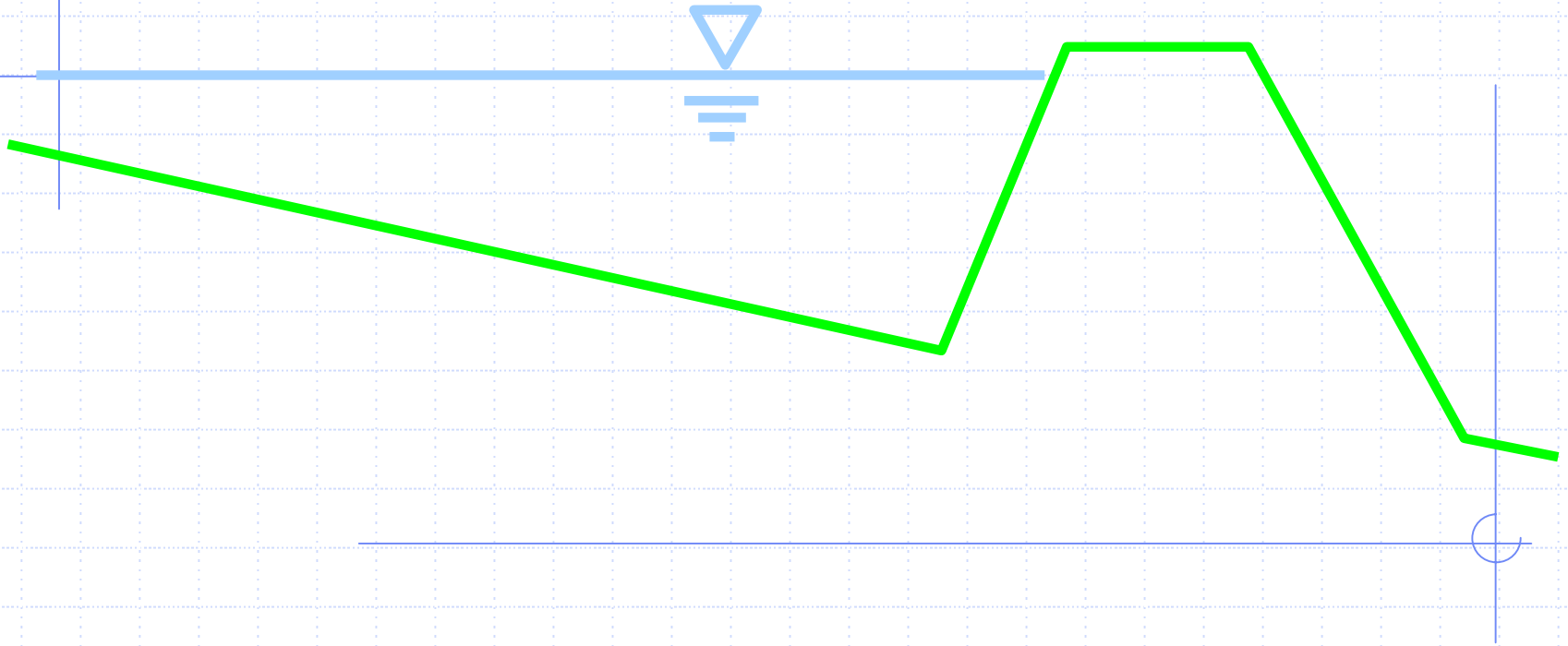


# City of Springfield

## Drainage Criteria Manual

### Detention



aM Hydro Division

# *Basic Concepts*

- ◆ *Detention Reservoir Design*
- ◆ *Basic concepts*
- ◆ *Sizing/Siting*
- ◆ *Routing/Outlet structures*
- ◆ *Examples*

# Basic Concepts

- ◆ *Continuity,  $Q_{in} - Q_{out} = dS/dT$*
- ◆ *Energy Grade Line - The total energy at any point in the system (sum of the elevation, pressure head and velocity head)*
- ◆ *Hydraulic Grade Line - The sum of the elevation and pressure head. The HGL is coincident with the water surface in free surface flow*

# Basic Concepts

- ◆ *Unit Hydrograph -The hydrograph resulting from one inch of constant intensity, uniformly distributed, excess rainfall*
- ◆ *Abstractions -Any portion of rainfall that does not become runoff such as infiltration, interception, evaporation, transpiration or depression storage*
- ◆ *Runoff Volume -The total volume of runoff expressed in acre-feet, inches over the watershed or cubic feet*

# A Note on Probability

- ◆ *A culvert with a design life of 50 years that is designed to carry a 50-year recurrence interval event has a 64% probability that its capacity will be equaled or exceeded during its design life*
- ◆ *For an improvement with a design life of 2 years, designed for a 2-year recurrence interval, the probability of failure increases to 75%*

# Basic Concepts

- ◆ *Time of Concentration - The time it takes for water to travel from the hydraulically most remote point in the watershed to the point of interest*
- ◆ *Routing - The process of accounting for the effects of hydrograph translation and attenuation due to channel or reservoir storage*

# Basic Concepts

- ◆ *Recurrence Interval - 1/annual probability of an event being equaled or exceeded, synonymous with return period*
- ◆ *Temporal Distribution - Distribution of rainfall with respect to time*
- ◆ *Tailwater - Depth of water downstream of the pipe, channel or structure of interest, excess tailwater may have backwater effects reducing the system capacity*

# Detention Design

- ◆ *Why detention?*
- ◆ *To meet regulations*
- ◆ *To mitigate some of the impacts of urbanization*

# Missouri Water Law

## Rule of Reasonable Use

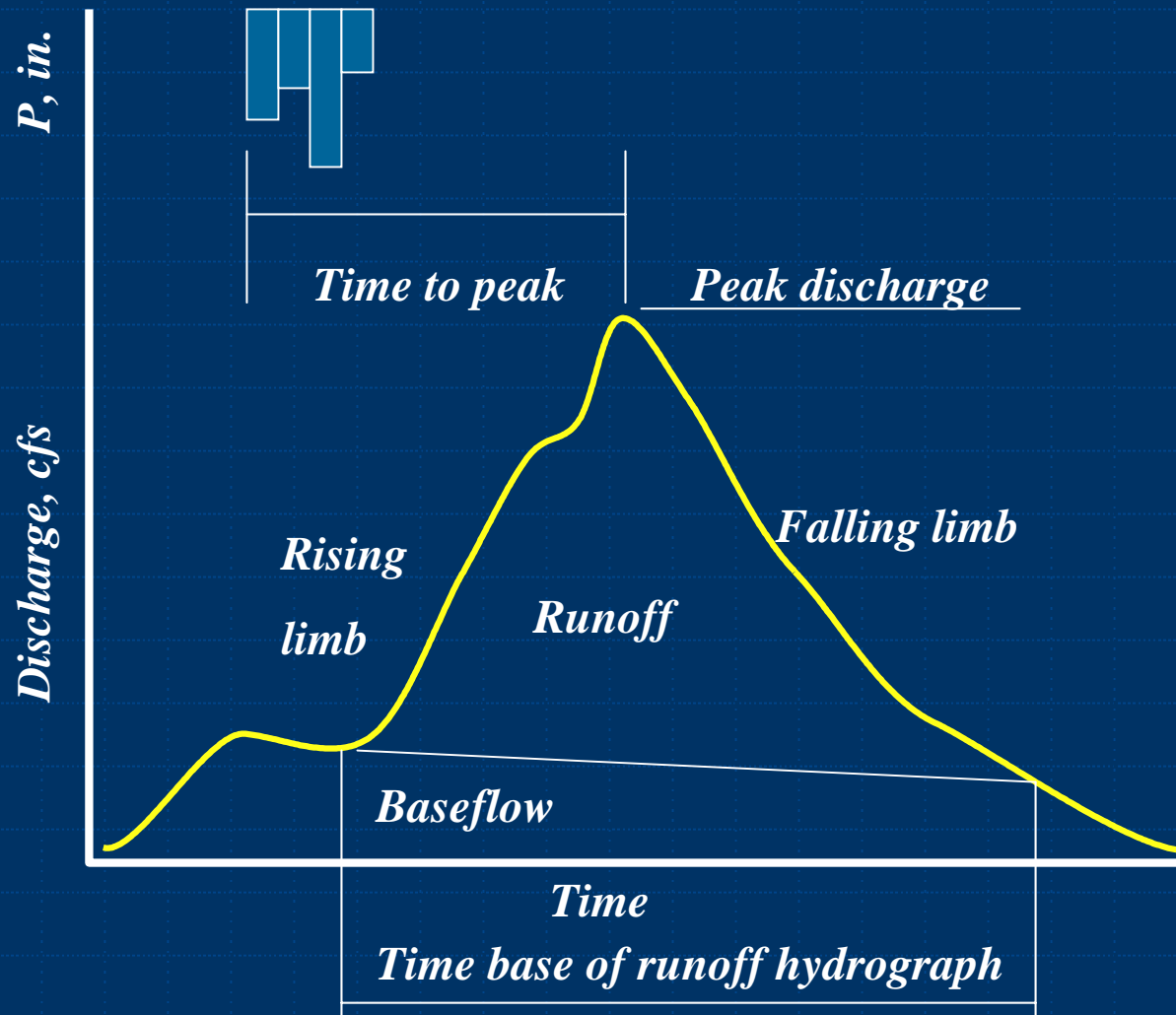
### ◆ *Three tenets necessary to apply the rule of reasonable use*

- ◆ *Was the alteration necessary for the use of the property?*
- ◆ *Were reasonable steps taken to mitigate any adverse impacts?*
- ◆ *Does the utility of the alteration outweigh any potential harm?*

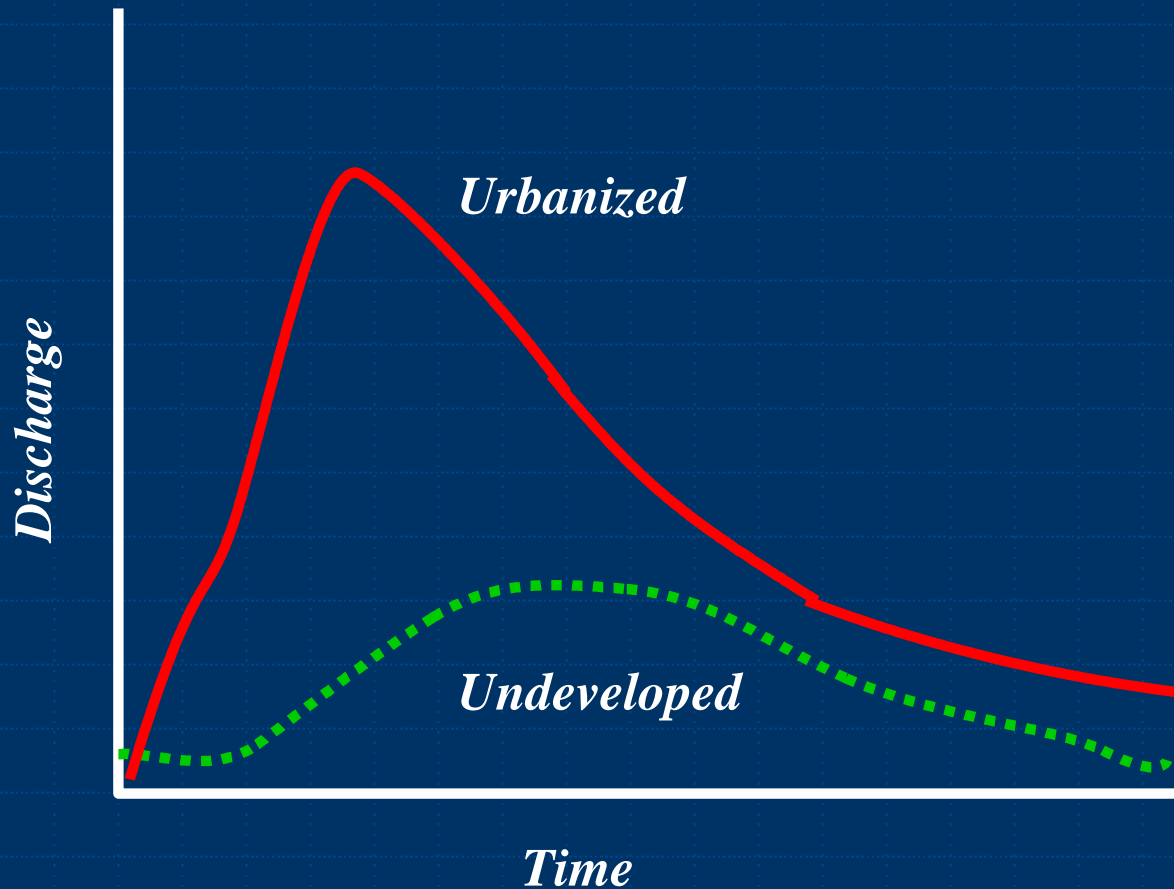
# Impacts of Urbanization

- ◆ *Greater volume of runoff*
- ◆ *Faster time to peak*
- ◆ *Greater peak discharge*
- ◆ *Reduction in dry weather baseflow*
- ◆ *Reduction in water quality*
- ◆ *Increase in water temperature*

# Runoff Hydrograph



# Impacts of Urbanization



# Types of Detention

- ◆ *Detention / Retention*
- ◆ *Wet / Dry*
- ◆ *On line / Off line*
- ◆ *Regional / Site specific*

# Types of Flood Control Storage

- ◆ *Detention temporarily stores runoff. Runoff either gravity drains or is pumped from the facility*
- ◆ *Retention stores runoff until it either evaporates or infiltrates*
- ◆ *Wet detention maintains a permanent pool*
- ◆ *Dry detention drains or is pumped dry*

# Types of Flood Control Storage

- ◆ *An on line facility is on the main drainage channel*
- ◆ *Off line detention is located off of the main channel and generally has a diversion structure to route runoff from the main stream to the reservoir*
- ◆ *Regional detention provides storage for several tracts of land*
- ◆ *Site specific detention requires each tract of land to provide detention as it develops*

# Regional Detention

## ◆ *Advantages -*

- ◆ *More effective flood control*
- ◆ *Requires watershed planning*
- ◆ *More cost effective*
- ◆ *Single large structure has less maintenance requirements than several small structures*

## ◆ *Disadvantages -*

- ◆ *Timing of construction*
- ◆ *Collection of fees*
- ◆ *Land availability in developed areas*
- ◆ *Requires watershed planning*

# Site Specific Detention

## ◆ *Advantages -*

- ◆ *Perceived equity in system*
- ◆ *Relatively easy to regulate*

## ◆ *Disadvantages -*

- ◆ *Multiple small structures pose a maintenance problem*
- ◆ *Area of effective flood control limited to immediately downstream of structure*

# Site Selection Issues

- ◆ *Proximity to flood prone areas*
- ◆ *Land availability*
- ◆ *Land ownership*
- ◆ *Gravity inflow & outflow*
- ◆ *Existing structures*
- ◆ *Lateral constraints*
- ◆ *Vertical constraints*

# 1.1 Detention Requirement

- ◆ "... post-development peak runoff rate for all land development must be no greater than the pre-development peak runoff rate for the design storms listed in this chapter."
- ◆ Basins must be safe, maintainable, enhance the environment and provide aesthetic value to the community
- ◆ Multipurpose use is encouraged

## 1.2 Payment in Lieu of Detention

- ◆ “engineer can show that constructing a detention basin will provide no downstream benefits and no existing downstream drainage problems will be impacted, a payment in lieu of constructing detention may be acceptable.”
- ◆ Requires signed and sealed application
- ◆ Does not waive water quality requirements

# 1.3 Design Spreadsheets

- ◆ Detention volume estimation
  - Modified FAA method
  - Hydrograph methods
- ◆ Stage-Storage computations for various geometries
- ◆ Outlet structure rating computations for various outlets

# 2.0 Types of Detention

- ◆ Dry
- ◆ Extended
- ◆ Wet
- ◆ In-Line
- ◆ Off-Line

# 3.0 Hydrologic & Hydraulic Design

## ◆ Table DET-1, Design Methods

<b>Detention Design Method</b>	<b>Acceptable Volume (cubic feet [ft<sup>3</sup>])</b>	<b>Approx. Acceptable Impervious Area Added</b>
Tabular Method (Section 3.1.1)	< 5,000 ft <sup>3</sup>	½ acre
Simplified (Modified FAA) Method (Section 3.1.2)	<20,000 ft <sup>3</sup>	2 acres
Hydrograph Method (Section 3.1.3)	Any size	Any amount

## 3.1.1 Tabular Method

- ◆ Applicable for volumes less than 5000 ft<sup>3</sup>.
- ◆ Criteria listed in Table DET-2
- ◆ Table DET-2 is also applicable to determine the required volume for payment in lieu of detention

# 3.1.1 Tabular Method Criteria

- ◆ Flow rates determined using Rational Method
- ◆ Low-flow orifice limited to 1-year peak flow rate, minimum 6" diameter
- ◆ Overflow spillway for 100-year flow rate ABOVE the required volume
- ◆ Turf reinforcement or sod preferred for overflow spillways
- ◆ 6 inches of freeboard above 100-year water surface required.

## 3.1.2 Simplified Method

- ◆ Applicable for volumes less than 20,000 cubic feet
- ◆ Modified FAA Method
- ◆ [Spreadsheet](#) available to assist with computations

## 3.1.3 Hydrograph Methods

- ◆ Detention basins larger than 20,000 cubic feet
- ◆ Methods
  - SCS Dimensionless Unit Hydrograph
  - Kinematic wave hydrograph generation

# 3.2.1 Peak Flow Attenuation

- ◆ 1-yr, 10-yr and 100-yr events must be analyzed
- ◆ Analysis conditions
  - Condition 1, Pre-project
  - Condition 2, Post-project
  - Condition 3, Fully urbanized w/no upstream detention unless previously approved (100-yr only)
- ◆ Downstream conditions may warrant spillway capacity greater than the 100-yr

## 3.2.3 Outlet Works Design

- ◆ Must restrict the post-project release rates to less than or equal to the pre-project rates
- ◆ Design frequencies 1-yr, 10-yr, and 100-yr
- ◆ Critical duration analysis required

## 3.3.1 Detention Volume Estimation

- ◆ Direct hydrograph subtraction
  - Typical final volume = 1.5 –2 times the direct subtraction volume
- ◆ Modified FAA Method
- ◆ Hydrograph Volumetric Method
- ◆ Rule of thumb 0.35 acre-feet of storage required for each acre of impervious area

## 3.3.2 Outlets

- ◆ Orifices

- ◆ Weirs

- Broad-crested
- Sharp-crested
- Vertical Slots
- Slots with sloping sides

- ◆ Culverts

- ◆ Risers

## 3.3.3 Reservoir Routing

- ◆ Level-pool/Modified Puls Routing
- ◆ Numerous computer programs available
- ◆ Suitable for spreadsheet computations

# 4.0 Final Design Considerations

- ◆ Potential for multiple use
  - Recreational facilities must be at or above the 2-year stage in the reservoir
- ◆ Basins to be located at the natural low point of the site
  - Must discharge to the natural drainage location

## 4.3 & 4.4 Grading and Geometry

- ◆ Utilize natural contours to the maximum extent practical
- ◆ Maximum slopes 4H:1V
- ◆ Minimum 1% grade in bottom of basin
- ◆ Concrete low-flow channel required for 2-year greater than 5 cfs
- ◆ Hard improvements required to control 2-yr through 10-yr discharge

## 4.3 & 4.4 Grading and Geometry

- ◆ Sod or turf reinforcement may be acceptable for velocities less than 4 ft/s
- ◆ Water quality grading should consider a gradual expansion at the inlet and gradual contraction at the outlet to avoid short-circuiting flow.

## 4.5 Embankments & Cut Slopes

- ◆ Maximum of 4H:1V
- ◆ Minimum of 1 ft. of freeboard between the top of embankment and the 100-yr water surface elevation
- ◆ Increase embankment heights by 5% to allow for settlement
- ◆ MDNR Dam Safety criteria may control

## 4.6 Linings

- ◆ Impermeable linings may be required if there is potential to increase ground water levels and adversely impact adjacent properties
- ◆ Permeable linings are encouraged where practical to encourage ground water recharge

## 4.7 Inlets

- ◆ Inlets to the detention facility must incorporate energy dissipation to reduce erosion
- ◆ Forebays or sediment traps are required at all inflow points.
- ◆ Access must be provided for maintenance of sediment traps

## 4.8 Outlet Works

- ◆ Outlets must be sized and designed to release the specified flow rates without structural or hydraulic failure
- ◆ Insufficient erosion protection can lead to structural failure
- ◆ Trash racks must not interfere with hydraulic capacity and must be protective of public safety

# 4.10 Vegetation

- ◆ Planting plan required
- ◆ Plantings function of:
  - Use
  - Frequency and duration of inundation
  - Soil types
  - Native or non-native plants
- ◆ Trees and shrubs are not recommended for dam or embankment slopes

## 4.11 Operation & Maintenance

- ◆ Consider landscaping that discourages entry
- ◆ Easements for access and maintenance are required
- ◆ Permanent ponds should have provisions for completely draining the facility for maintenance
- ◆ Permanent pools may require aeration to minimize odors and other nuisances
- ◆ Secondary uses that are incompatible with sediment deposits should not be planned unless maintenance provisions are included

## 4.11 O & M cont'd

- ◆ French drains are discouraged in areas where high sediment loads are expected
- ◆ Underground detention must provide multiple access points of sufficient size for entry to remove sediment and debris
- ◆ Permanent pools should be of sufficient depth as to discourage excessive aquatic vegetation
- ◆ Trash rack designs should consider aesthetics and maintenance

## 4.11 O & M cont'd

- ◆ To minimize maintenance, outlet structures should be designed with no moving parts
- ◆ The use of sharp plates for weirs is discouraged due to potential for damage, vandalism and accidents
- ◆ Forebays and sediment traps are to be cleaned on a regular basis, at a minimum when the trap is  $\frac{3}{4}$  full of sediment

## 4.12 Access

- ◆ All-weather, stable access to the bottom, inflow, forebay, and outlet works areas shall be provided for maintenance vehicles
- ◆ Maximum grades should be 10 percent, and a solid driving surface of gravel, rock, concrete, or gravel-stabilized turf should be provided

## 4.13 Geotechnical

- ◆ Geotechnical considerations may include issues related to embankment stability, geologic hazards, seepage, and other site-specific issues
- ◆ It may be necessary to confer with a qualified geotechnical engineer during both design and construction, especially for larger detention and retention storage facilities.

# 4.14 Environmental Permitting

- ◆ U.S. Army Corps of Engineers 404 permit
  - Wetlands
  - Waters of the United States
- ◆ Missouri Department of Natural Resources 401 Water Quality Certification
- ◆ NPDES permit